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# Pets Return Home Site Design

Ruff Engineering

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CENE 486 Final Presentation



April 24, 2020

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# Purpose

## CREATE:

- Site design for expansion of the kennel space
- Drainage plan for sanitary sewer runoff

# Location

4555 N. Peyton Place

**City:** Clarkdale

**County:** Yavapai County

**State:** Arizona

# Client

- Mark Happe: Co-founder of Pets Return Home Sanctuary



Figure 1: Aerial view of site location [1].

# Zoning Due Diligence

Zoning ordinances considered applicable in relation to the project are as follows:

## Yavapai County Designation

- RCU - Includes Rural, Single-family, residences
- Allow uses of R1L, RMM, and R1 Districts
  - R1L - single family residences limited to site built structures
  - RMM - single family, residential properties with site built, factory built and multi-sectional manufactured homes, no single-wide manufactured homes
  - R1 Districts - single family, residential properties with sit ebuilt, multi-sectional and manufactured structures

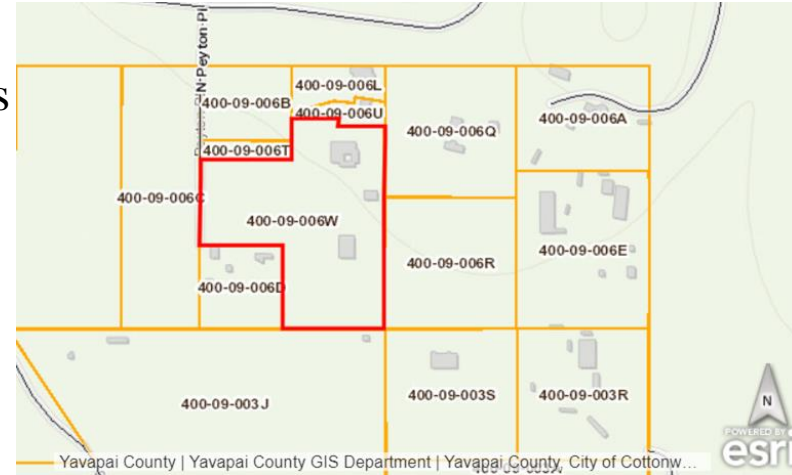


Figure 2: Parcel number and location map provided by the Yavapai County Interactive Map [2].

# Geotech Field Investigation

*Prior to site visit AZ 811 was contacted and a Safety and Sampling Plan were created.*

In-situ data collection performed at all locations (Fig. 4)

- Test pit log of observed soil
- Grab samples of each observed soil type
- Ring samples

Figure 3 : Image of ring sample collected preserving the in-situ conditions of the soil.



Figure 4: Testing Locations.

# Geotech Field Investigation - Infiltration Test

- Infiltration tests at location 2, 3, and 4 at approx. 4 ft deep
- Performed according to ADEQ - R18-9-A310 - subsection F
- Infiltration test results range from 16 to 68 minutes per inch of water infiltrated

Figure 5: Typical test pit after excavation (right) and a test pit with ongoing infiltration test (left).



Infiltration Rates			
Test Pit Location	2	3	4
Percolation (mins./inch)	68	56	16

Table 1: Results of infiltration tests for all site locations.

# Geotechnical Lab Analysis

## Tests performed:

- Soil classification - ASTM D2487
- Hydrometer - ASTM D7928-17
- Field moisture contents - ASTM D2216
- In-situ soil density - ASTM D2937
- Remolded expansion potential - ARIZ 249
- Compression - ASTM D2435
- Liquid limit and plasticity index - ASTM D4318-17el.
- Moisture density relationship/proctor - ASTM D698-12e2

Sample	1(0-2)	2(0-4)	2(PERK)	3(0-2)	3(PERK)	4(0-3)	4(3-4)	4(PERK)
	<b>Soil Classification</b>							
<b>Replicate 1</b>	SC-SM	SC	SC	SC	SC	CL	SC	SC-SM
<b>Replicate 2</b>	SC-SM	SC	SC	SC	SC	CL		SC-SM
<b>Replicate 3</b>	SC	SC	CL	SC	SC	CL		SC-SM
<b>Final</b>	<b>SC-SM</b>	<b>SC</b>	<b>SC</b>	<b>SC</b>	<b>SC</b>	<b>CL</b>	<b>SC</b>	<b>SC-SM</b>

Table 2: Soil classification results from samples taken at Locations 1, 2, 3, and 4.

# Lab Analysis - Remolded Expansion Potential

The expansion percentages that are seen in Figure (left) are in the zero swell potential, 0% to 1.5%, and moderate swell potential, 1.5% to 3%.

REMOLDED SWELLS							
TEST PIT NO.	DEPTH (FEET)	SOIL CLASSIFICATION	SOIL PROPERTY		EXPANSION		REMARKS
			INITIAL DRY DENSITY (PCF)	INITIAL WATER CONTENT (%)	SURCHARGE (KSF)	EXPANSION (%)	
1	0-2	SC-SM	112.2	10	0.1	2.3	1,2
2	0-4	SC	113.3	12.7	0.1	0	1,2
2	4-5	SC	113.3	12.7	0.1	0.6	1,2
3	0-2	SC	113.3	12.7	0.1	0.2	1,2
3	4-5	SC	111.3	13.5	0.1	0.2	1,2
4	0-3	CL	113.3	12.7	0.1	1.8	1,2
4	3-4	SC	111.3	13.5	0.1	0.7	1,2
4	4-5	SC-SM	113.3	12.7	0.1	0.1	1,2

Remarks:

1. Compacted Density (approximately 95% of ASTM D698 maximum density and -3% below optimum moisture content)
2. Submerged to approximate saturation

Table 3: Remolded Swells initial conditions and final swell potential results.

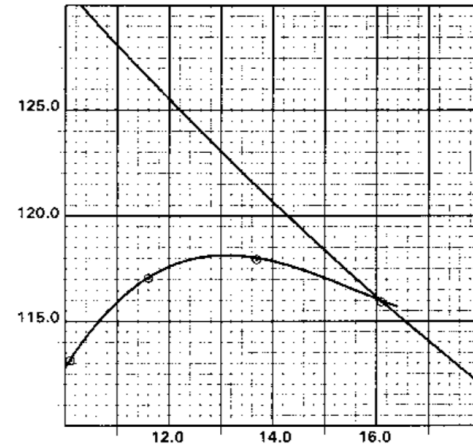


# Lab Analysis - Compaction Proctor

Maximum density = 118.1 lbs/ft<sup>3</sup>

Optimum moisture content = 13.0%

If no additional soil is used to produce grade under proposed kennel this data can be used to compare field density to determine rate of compaction and moisture content compliance.



Maximum Density	<u>118.1</u>	lbs/ft <sup>3</sup>	Optimum Moisture	<u>13.0</u>	%
Computed Max	<u>118.1</u>	lbs/ft <sup>3</sup>	Computed Opt	<u>13.0</u>	%
ASTM Corr Max	<u>118.1</u>	lbs/ft <sup>3</sup>	Corr Opt	<u>13.0</u>	%
AASHTO Corr Max	<u>118</u>	lbs/ft <sup>3</sup>	Corr Opt	<u>13</u>	%
ARIZ Corr Max	<u>118.1</u>	lbs/ft <sup>3</sup>	Corr Opt	<u>13.0</u>	%

Figure 6: Compaction proctor results (unit weight and optimum moisture).



# Field Investigation - Existing Slab

## Existing Slab Investigation Results:

- 4-5 inch thick slab-on-grade
- No Foundation
- Undermined Base
- Underlying soils in moist to wet conditions



Figure 7: Measurement of slab thickness

# Slab on Grade Analysis

Meyerhof's shallow foundation was observed to determine the bearing capacity of the existing surface (see Equations left).

Results show:

Net ultimate bearing capacity = 21,000 lb

Factor of Safety = 3

Net stress = 7,000 pounds.

*Equation 1: Meyerhof Shallow Foundation Bearing Capacity*

$$q_u = c'N_cF_{cs}F_{cd}F_{ci} + qN_qF_{qs}F_{qd}F_{qi} + \frac{1}{2}\gamma B N_\gamma F_{\gamma s}F_{\gamma d}F_{\gamma i}$$

$q_u$  = Net ultimate bearing capacity

$c'$  = Cohesion

$q$  = effective stress at the level of the bottom of the foundation

$\gamma$  = unit weight of soil

$B$  = width of foundation

$F_{cs}$ ,  $F_{qd}$ ,  $F_{\gamma s}$  = shape factors

$F_{cd}$ ,  $F_{qd}$ ,  $F_{\gamma d}$  = depth factors

$F_{ci}$ ,  $F_{qi}$ ,  $F_{\gamma i}$  = load inclination factors

$N_c$ ,  $N_q$ ,  $N_\gamma$  = bearing capacity factors

*Equation 2: The Gross Allowable Load*

$$q_{all} = q_u / FS$$

$q_{all}$  = Net stress increase on soil

$q_u$  = Net ultimate bearing capacity

FS = Factor of safety

# Surveying

Equipment used:

- Nikon Total Station
- Rod and Prism
- Nomad Data Collector
- Tripod



Figure 8: Topographic Map of site.



Figure 9: Septic tank and concrete pad location on site.

# Hydrology

- Flow Routing
  - Contours suggest flow seen in Figure 10
- Weighted Curve Number
- Time of Concentration
- Storm Event Runoff
  - Yavapai County Drainage Design Manual

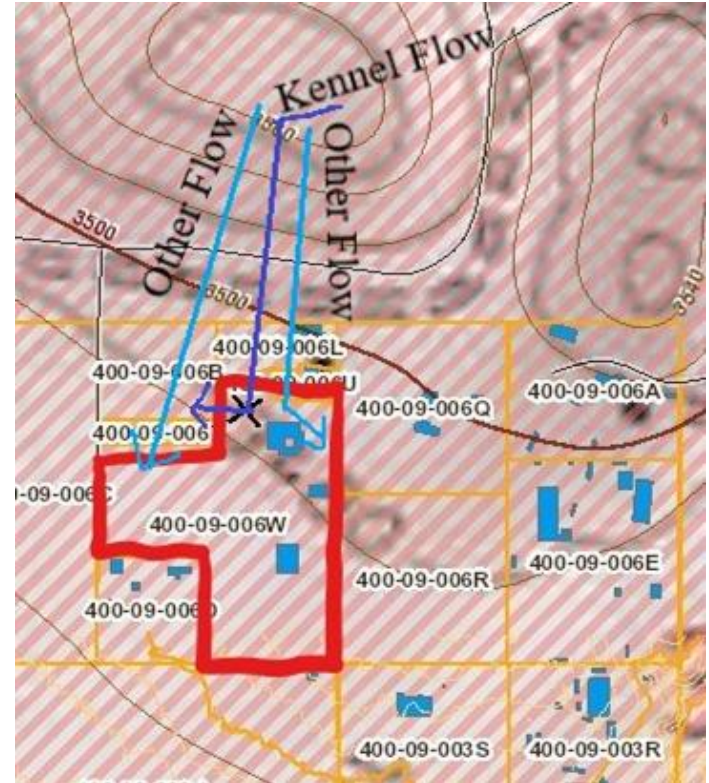


Figure 10: Flow Routing

# Hydrology

Table 4: Weighted Curve Number

Percentage of Surface Type within Sub-Basin (%)				Weighted C
Natural Desert Rangeland	Hillslopes	Gravel Road	Roof	
66%	16%	16%	2%	0.58
0.48	0.67	0.84	0.95	
<b>Runoff Coefficient</b>				

Table 5: Time of Concentration

<b><i>Time of Concentration</i></b>	<b><i>30 min</i></b>
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Table 6: Storm Event Runoff

Flow Through Kennels	
Storm (yr)	Q (cfs)
1	0.57
2	0.74
5	1.00
10	1.21
25	1.53
50	1.79
<b>100</b>	<b>2.07</b>

# Decision Matrix

Criteria weight based on ability to affect the client’s suggested importance.

Design’s ranked; “one” being the design that best met the criteria and “three” being the design that least met the criteria.

Criteria weight and design rank were multiplied and summed together to give a weighted score for each design.

Table 7: Decision Matrix

	Decision Criteria				
	Sanitation	Area Required	Construction Cost	Maintenance Cost	
Weight	23.00%	23.00%	31.00%	23.00%	Score
Septic Tank and Leach Field	1	1	2	2	1.54
Lagoon	3	2	1	1	1.69
LID Retention Pond	2	2	3	1	2.08

\*Lowest score means highest expectation.

# Septic Tank Storage Determination

Utilized Bernoulli's Energy Equation

Assumptions made:

- Assumed 200 ft PVC pipe from well to facet at hose
- Hose length is 100 ft
- Elevation change from pump to pad is little to none - assumed zero

Flow rate of 3.4 gpm found

Client washes pad for 1 hour daily; utilizing 200 gallons per day

ADEQ R18-9-A314 suggests minimum design capacity be 1000 gallons

Equation 3: Bernoulli's Equation

$$\frac{P_1}{\gamma} + \frac{V_1}{2g} + Z_1 + h_p = \frac{P_2}{\gamma} + \frac{V_2}{2g} + Z_2 + h_f$$

With:

$P_n$  = Pressure

$V_n$  = Velocity

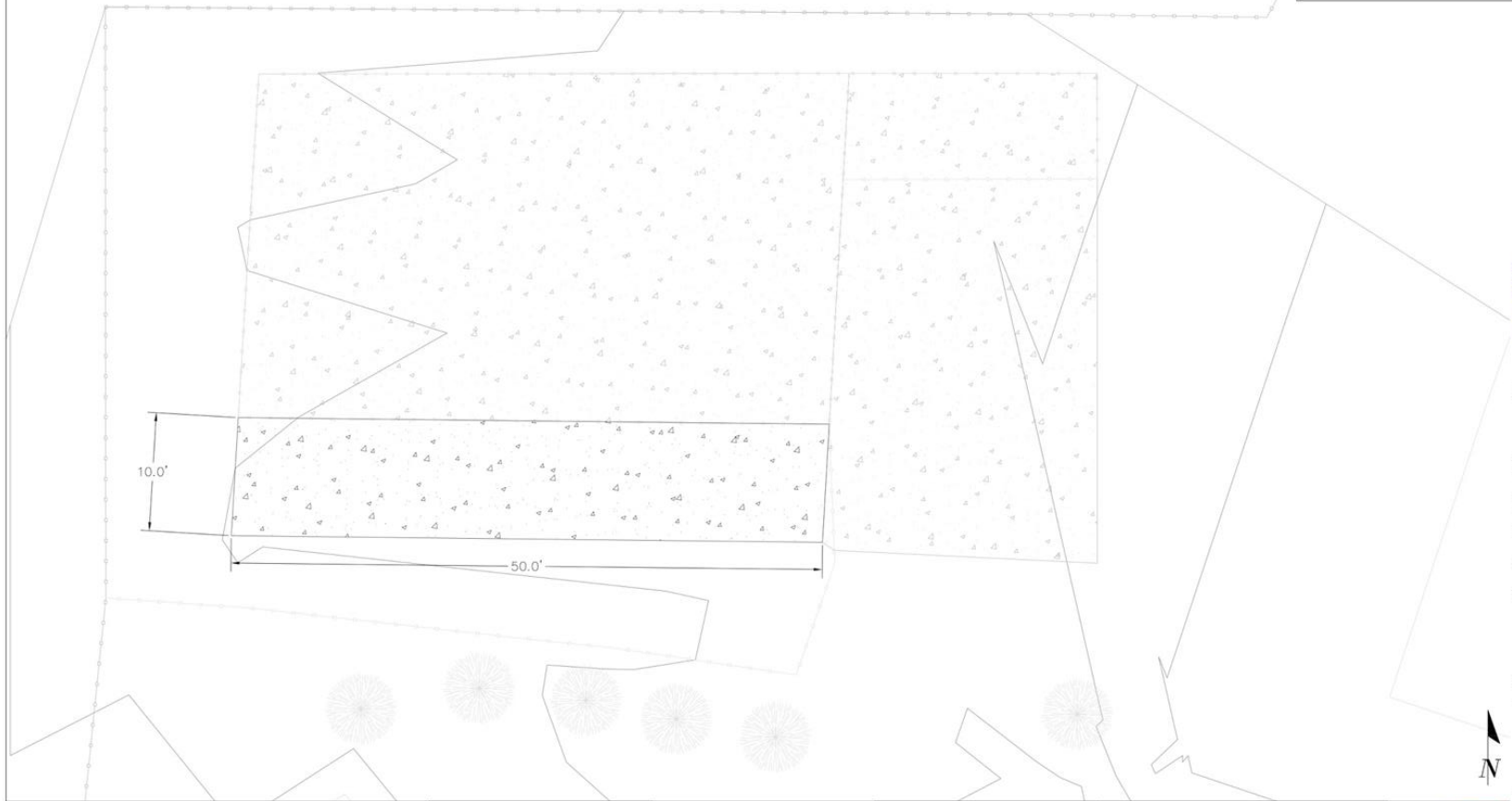
$Z_n$  = Elevation

$h_p$  = Head at Pump

$h_f$  = Head loss due to Friction



QUANTITIES  
9.5CYD CONCRETE



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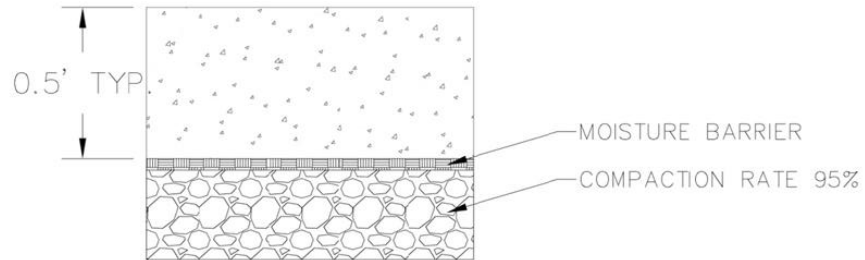
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CONCRETE SLAB CROSS-SECTION TYP.  
N.T.S

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# DETAILS

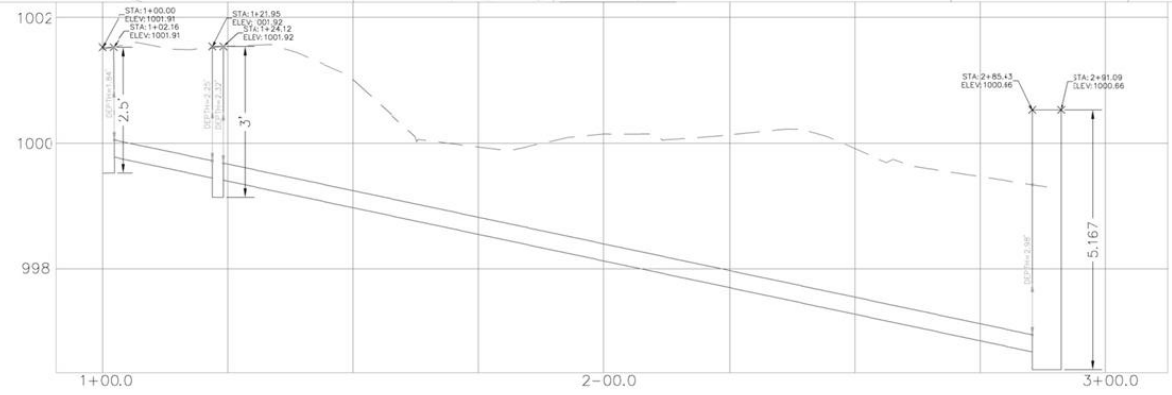
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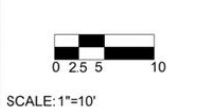
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**PLAN AND PROFILE**

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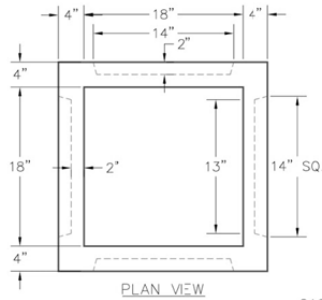


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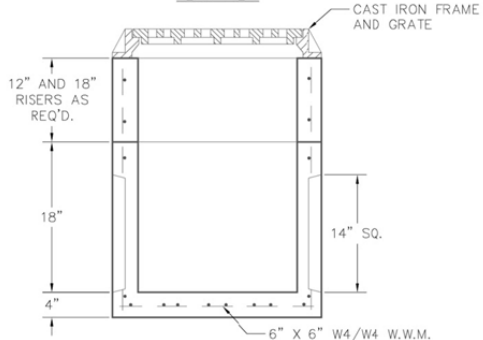
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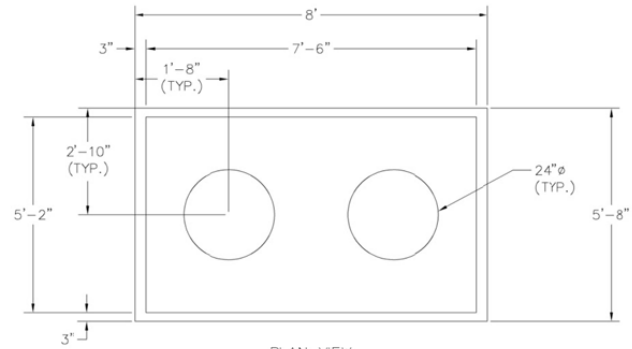
PLAN VIEW



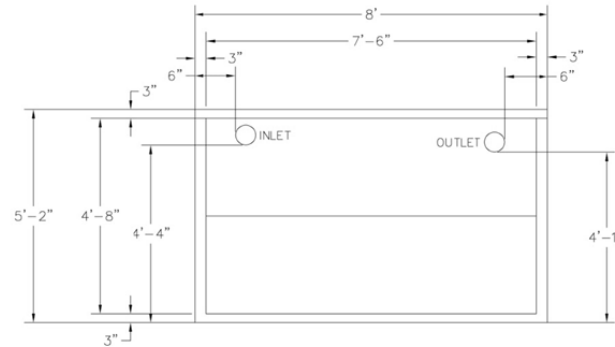
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NOTE:  
 CONCRETE: 4,000 PSI @ 28 DAYS  
 REINFORCING: AS PER ASTM A-185 6"X6"  
 W4/W4 W.W.M.  
 WEIGHTS: CATCH BASIN - 645 LBS. CONCRETE  
 FLAT TOP ALSO AVAILABLE 180 LBS. (3" THICK)  
 RISER WEIGHTS: 363 LBS./FT.

CATCH BASIN  
 N.T.S



PLAN VIEW



SECTION

SEPTIC TANK  
 N.T.S

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# DETAILS

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# Final Recommendations

- Construction
  - Expand 10 feet south
  - Tie into existing surface
  - 95% compaction of ASTM D698, and +/- 3% of optimum moisture
  - Add a moisture barrier
  - Pad thickness 5 inches
- Drainage
  - Add two catch basins at the low points
  - 4 inch PVC pipe
  - Septic tank:
    - Width: 8 feet
    - Depth: 5 feet 8 inches
    - Height: 5 feet 2 inches
    - Volume: 1,000 gallons
  - Leach field:
    - Rows: 7
    - Width: 2 rows 5 foot, 5 rows 10 foot
    - Length: 25 feet
    - Total Area: 1,500 feet <sup>2</sup>

# Impacts of Design

- Social
  - More room resulting in increased exercise and mental welfare of dogs
  - Little to no impact on work load for client and volunteers
  - Increased health and safety of dogs and people
- Environmental
  - No more septic pools at the end of the kennel
  - Lower the amount of water flow into the verde river
  - Micro dust particles into the air
  - Lower water flow affecting plant growth

- Economical
  - Dogs adopt-ability increases
  - Increase revenue
  - Decreasing infection/illness expenses
  - Additional revenue needed to cover cost of construction/maintenance



Figure 11: Sleepy puppies after a hard day of work.  
Photo Credit: Abigail Autieri

# Cost of Design

Table 8: Quantity and Cost

<b>Materials</b>	<b>Unit price</b>	<b>Units</b>	<b>Total</b>
Vapor Barrier (\$/per unit)	\$60.00	1	\$60.00
Cement (\$/per bag)	\$4.55	312.5	\$1,421.88
1,000 gal Septic Tank (\$/per tank)	\$1,000.00	1	\$1,000.00
4 inch PVC pipe (\$/per 10 feet length)	\$20.00	18.5	\$370.00
Steel frame for catch basin (\$/per unit)	\$240.00	2	\$480.00
Septic Tank Installation (\$/per tank)	\$5,000.00	1	\$5,000.00
<b>Total Cost</b>			<b>\$8,331.88</b>



# References

- [1] Google. “4555 N. Peyton Place in Clarkdale, Arizona” [Online]. Available: <https://goo.gl/maps/oGF4dUhMb2ud5J6s8>. [Accessed: October 6, 2019].
- [2] Y. C. GIS, “Interactive Map,” *Yavapai County Interactive Map*. [Online]. Available: <http://gis.yavapai.us/V4/map.aspx?zoom=3&x=-112.41532745361118&y=34.780708973222005&layers=Parcels,ParcelLabels,MajorRds,MajorRdLabels,Roadctrline,RdLabels,CityBnds,Cities,CityLbl,CountyBdy,CountyLbl,ChiZon,ChiZonLbIs>. [Accessed: 13-Jan-2020].
- [3] *Planning and Zoning Ordinance For The Unincorporated Areas of Yavapai County, Arizona*. Yavapai County Board of Supervisors, 2003.

# Questions?



Figure 12: Dr Bero with Angel.  
Photo Credit: Ryann DuBose



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